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# FORCED VIBRATION OF TIMOSHENKO BEAMS MADE OF MULTIMODULAR MATERIALS

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### FORCED VIBRATION OF TIMOSHENKO BEAMS MADE OF MULTIMODULAR MATERIALS1

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### 1 INTRODUCTION

Many materials have different elastic behavior in tension and compression. A few examples of such materials are concrete, rock, tire-cord rubber, and soft biological tissues. As early as 1864, St. Venant [1] recognized this behavior by analyzing the pure bending behavior of a beam having different stress-strain curves in tension and compression. Timoshenko [2] originated the concept of bi-modulus (or bimodular) materials in 1941 by considering the flexural stresses in such a material undergoing pure bending. Ambartsumyan [3] in 1965 renewed interest in the analysis of bimodular materials, i.e., materials having different moduli in tension and compression. Since then, there have been numerous investigations on the static behavior of bimodular beams; these were surveyed by Tran and Bert [4]. Recently, Bert and Gordaninejad [5] studied bending of thick beams of "multimodular" materials.

Only a few studies have been made on vibration of bimodular beams. Recently, Bert and Tran [6] worked on transient response of thick beams of bimodular materials.

The present paper deals with the forced vibration of beams made of "multi-modular" materials. The transfer-matrix method [7], which computationally is very efficient, is applied. Also, the beam is modeled as a Timoshenko beam, i.e., both transverse shear deformation and rotatory inertia are considered.

### 2 MODELING OF THE STRESS-STRAIN CURVE

The nonlinearity of the stress-strain curve is one of the main difficulties arising in structures undergoing even moderate deflections. Piecewise linearization of the stress-strain relation has been applied to overcome this problem.

Durban and Baruch [8] used a floating piecewise linear approximation to construct the two "best" straight lines approximating the Ramberg-Osgood stress-strain relation [9]. Bert and Kumar [10] recently presented experimental stress-strain curves

for unidirectional cord-rubber materials and expressed the curves in Ludwik power-law form with different coefficients and exponents in tension and compression.

In the present work, a stress-strain curve for aramid-rubber taken from [10] has been linearly approximated by four segments (two segments in tension and two segments in compression; see Fig. 1). For choosing the "best" two straight lines, the area between two fitting lines and the experimental curve in each portion has been minimized. To find comparable moduli for the bimodular case, one has to minimize the area between two straight lines and the experimental curve. Finally, for the "unimodular" case, the "best" single straight line is used (see Appendix A).

### 3 CLOSED-FORM SOLUTION

Consider a solid rectangular-cross-section beam of thickness h and length 2. The beam coordinates are taken such that the xy-plane coincides with the mid-plane of the beam and the z-axis is measured positive downward. For a four-segment approximation of the normal stress-strain curve, considering the general case (i.e., when -  $h/2 < a_c$ ,  $a_t < h/2$ ), the following stress field has been considered for the case of convex bending (see Figs. 1 and 2).

$$\sigma_{x} = \begin{cases} E_{1}^{c} \epsilon_{1}^{c} + E_{2}^{c} (\epsilon_{x} - \epsilon_{1}^{c}) & -h/2 < z < a_{c} \\ E_{1}^{c} \epsilon_{x} & a_{c} < z < z_{n} \\ E_{1}^{t} \epsilon_{x} & z_{n} < z < a_{t} \\ E_{1}^{t} \epsilon_{1}^{t} + E_{2}^{t} (\epsilon_{x} - \epsilon_{1}^{t}) & a_{t} < z < h/2 \end{cases}$$

$$(1)$$

$$\tau_{xz} = G_{Y_{xz}} \tag{2}$$

where  $E_1^c$ ,  $E_2^c$ ,  $E_1^t$ ,  $E_2^t$ , G,  $e_1^c$ , and  $e_1^t$  are material constants,  $\sigma_x$  is the axial normal stress,  $e_x$  is the axial normal strain,  $\tau_{xz}$  is the transverse shear strain,  $\tau_{xz}$  is the transverse shear stress, and  $z_n$  is the location of the neutral surface.

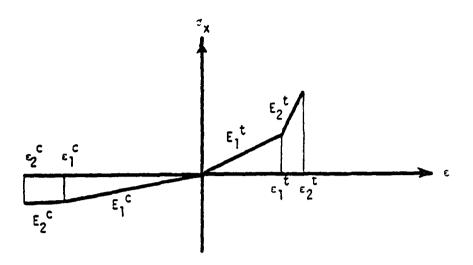


Fig. 1 Multimodular model.

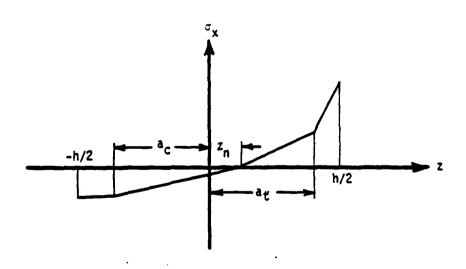


Fig. 2 Stress Distribution of a Multimodular Beam for Convex Bending

It is noted that this material is linear elastic in shear. Comparison of Figs. 1 and 2 leads to

$$\varepsilon_{x} = \kappa(z - z_{n}) \tag{3}$$

$$\varepsilon_1^{\ C} = \kappa(a_C - z_n) \tag{4}$$

$$\varepsilon_1^{t} = \kappa(a_t - z_n) \tag{5}$$

Using linear strain measure, one obtains

$$\varepsilon_{x} = U_{,x} = u_{,x} + z\psi_{,x}$$

$$\gamma_{xz} = W_{,x} + U_{,z} = W_{,x} + \psi$$
(6)

Comparison of equations (3) and (6) gives

$$u_{x} = -\kappa z_{n}$$
,  $\psi_{x} = \kappa$  (7)

Note that ( ) x denotes  $\partial(x)/\partial x$ .

Timoshenko beam theory is implemented here, by using the definitions of the normal and transverse shear stress resultants and moment, each per unit width as follows:

$$(N,Q) = \int_{-h/2}^{h/2} (\sigma_{x}, \tau_{xz}) dz , \quad M = \int_{-h/2}^{h/2} z \sigma_{x} dz$$
 (8)

One can write the constitutive relation for a multimodular beam as

$$\begin{cases}
N \\
M \\
Q
\end{cases} = \begin{bmatrix}
A + C_{N}^{A} & B + C_{N}^{B} & 0 \\
B + C_{M}^{B} & D + C_{M}^{D} & 0 \\
0 & 0 & S
\end{bmatrix}
\begin{cases}
u_{,x} \\
\psi_{,x} \\
w_{,x} + \psi
\end{cases} = \begin{bmatrix}
A' & B' & 0 \\
B'' & D' & 0 \\
0 & 0 & S
\end{bmatrix}
\begin{cases}
u_{,x} \\
\psi_{,x} \\
\psi_{,x} + \psi
\end{cases} (9)$$

where A, B, D, and S denote the respective extensional, flexural-extensional coupling, flexural, and transverse shear stiffnesses defined by

$$(A,B,D) = \int_{-h/2}^{h/2} (1,z,z^2) E_i^{(k)} dz \quad i=1,2 \\ k=t,c \quad S = K^2 \int_{-h/2}^{h/2} G dz$$
 (10)

Here, the stiffnesses  $C_N^A$ ,  $C_N^B$ ,  $C_M^B$ , and  $C_M^D$  are not present in unimodular or bimodular materials (see Appendix B). In equation (10), t and c denote tensilestrain and compressive-strain regions, respectively, and  $K^2$  is the shear correction coefficient. The general equations of motion, if  $z_n$  (neutral-surface location) is constant along the beam are

$$A^{1}u_{,xx} + B^{1}\psi_{,xx} = Pu_{,tt} + R\psi_{,tt}$$
  
 $S(w_{,xx} + \psi_{,x}) = Pw_{,tt} - q(x,t)$  (11)  
 $(B^{"}u_{,xx} + D^{"}\psi_{,xx}) - S(w_{,x} + \psi) = Ru_{,tt} + I\psi_{,tt}$ 

where

$$(P,R,I) = \int_{-h/2}^{h/2} \rho(1,z,z^2) dz$$

and  $\rho$  is the density of material.

For guided-guided boundary condition, i.e.,

$$u(0,t) = u(\ell,t) = 0$$
;  $\psi(0,t) = \psi(\ell,t) = 0$  (12)  
 $Q(0,t) = Q(\ell,t) = 0$ 

if

$$q(x,t) = q_0 \cos \alpha x \cos \alpha t \qquad (13)$$

then the following sets of functions satisfy the equations of motion

$$u(x,t) = \overline{U} \sin \alpha x \cos \Omega t$$
;  $\psi(x,t) = \overline{\Psi} \sin \alpha x \cos \Omega t$  (14)  
$$w(x,t) = \overline{W} \cos \alpha x \cos \Omega t$$

In actuality, enforcement of the axial-free equilibrium equation of elasticity requires that  $K^2$  for a multimodular beam (even a single-layer one, such as treated here) be a function of the level of normal strain (through the piecewise segmentation of the stress-strain curve). However, in this paper,  $K^2$  is assumed to be a constant.

where

$$\Omega = 2\pi f$$
 ,  $\alpha = m\pi/\ell$  (m = 1,2,3,...) (15)

f = circular frequency of the excitation, and

$$\bar{\mathbf{U}} = \frac{\mathbf{q_0}(S\alpha)(B'\alpha^2 - R\Omega^2)}{(S\alpha^2 - P\Omega^2)[B'\alpha^2 - R\Omega^2)(B''\alpha^2 - R\Omega^2) - (D\alpha^2 + S - I\Omega^2)(A'\alpha^2 - P\Omega^2)] + (S\alpha)^2(A'\alpha^2 - P\Omega^2)}$$

$$\bar{\mathbf{\Psi}} = \frac{\mathbf{A}'\alpha^2 - P\Omega^2}{\mathbf{B}'\alpha^2 - R\Omega^2} \quad \bar{\mathbf{U}} \quad ; \quad \bar{\mathbf{W}} = \frac{1}{S\alpha^2 - P\Omega^2} \left[\mathbf{q_0} - \frac{(S\alpha)(A'\alpha^2 - P\Omega^2)}{B'\alpha^2 - R\Omega^2}\right] \quad \bar{\mathbf{U}}$$
(16)

Since from equations (7)

$$z_n = -u_{,x}/\psi_{,x} \tag{17}$$

then

$$z_{n} = \frac{B'_{\alpha}^{2} - R_{\Omega}^{2}}{A'_{\alpha}^{2} - P_{\Omega}^{2}} = constant$$
 (18)

### 4 TRANSFER-MATRIX SOLUTION

The transfer-matrix model used in the present study is the same as that employed in [5] except for the station matrix (see Appendix C), which here includes more terms due to the motion. The transfer matrix for the assumed beam is of the following form

$$[S]_{N_{S}+1} = [T_{f}]_{\Delta \ell/2} [T_{S}] \prod_{i=1}^{N_{S}-1} \{ [T_{f}]_{i} [T_{S}]_{i} \} [T_{f}]_{\Delta \ell/2} [S]_{0}$$
(19)

where  $[T_f]_i$  is the field matrix,  $[T_s]_i$  is the station matrix,  $N_s$  is the number of stations,  $\Delta\ell/2$  is the length of each of the half fields at the ends of the beam,  $\Delta\ell$  is the length of each of the whole fields, and  $[S]_{N_S+1}$ ,  $[S]_0$  are state vectors, i.e.,  $(u,w,\psi,N,Q,M)^T$ , at the two ends of the beam.

In the calculation of the stiffnesses for the cases where the axial force is not zero, the neutral-surface location and the corresponding distances to the "break points" ( $a_c$  and  $a_t$ ) in the  $\sigma_x$  vs z curve are not constant and not known a-priori. Therefore, an iterative technique has been employed to compute the neutral-surface locations  $z_n$ , also  $a_c$  and  $a_t$ . One must first assume ( $2N_s + 2$ )

sets of values of  $z_n$ ,  $a_c$ , and  $a_t$  and then compute the sitffnesses and solve the governing equations for the state vector. Finally, by using equations (C.1), (C.3), and (C.4), compute the new values of  $z_n$ ,  $a_c$ , and  $a_t$ . Obviously, if the assumed and computed sets of  $z_n$ ,  $a_c$ , and  $a_t$  are in sufficiently close agreement, the problem is solved; otherwise, assume the calculated set  $z_n$ ,  $a_c$ , and  $a_t$  and repeat the procedure.

### 5 NUMERICAL RESULTS

The numerical results are presented for a thick, multimodular beam with a rectangular cross section. The material of the beam is chosen to be aramid cord-rubber which is used in automobile tires (see Table 1). Four different boundary conditions are investigated (see Table 2) and comparisons are made between multimodular, bimodular, and unimodular models for each set of boundary conditions. In this study, a mesh of twenty-five elements is used with each element being of length of 0.32 in. The shear correction coefficient is taken to be 5/6.

In order to validate the transfer-matrix solution (TMS), Fig. 3, a comparison is made between the closed-form solution (CFS) and the TMS for a guided-guided beam with cosine load distribution (case 1). Also, a comparison is made among unimodular, bimodular, and multimodular (static and dynamic) cases (see also Table 3) for f = 100 Hz. As one can see, there is excellent agreement between the TMS and CFS results. However, this agreement can be improved even further by increasing the number of elements.

For the other cases (2-4), the CFS is not available; therefore, in Figs. 4, 6, and 8 comparisons between different models (one, two, and four segment approximations) are made. As one might notice in all four cases, there is considerable difference between transverse deflection of multimodular and bimodular beams on one hand and that of the unimodular model on the other hand. In contrast, there is no substantial difference between multimodular and bimodular results.

Another interesting observation in Fig. 4 is that for f = 100 Hz, the unimodular beam is in the range of its first mode, whereas the bimodular and multi-

Table 1 Elastic properties and geometric parameters for an aramid-cord rubber beam

	Longitudinal Young's Modulus, MPa (psi x 10 <sup>-6</sup> )				Londitudinal Modulus, MPa	-Thickness Shear (psi x 10 <sup>-3</sup> )	
	Mode1*	Tens	ion	Compre	ession	Tension	and Compression
ies		E2t	E <sub>1</sub> t	E <sub>1</sub> C	E <sub>2</sub> c		G
Properties	М	4000 (0.580)	2896 (0.420)	221 (0.032)	71 (0.01)	3.70	(0.537)
ر ا		E	t b	E	c b		
Elasti	В		40 470)		124 .018)	3.70	(0.537)
<u> </u>	Ε			E			
	U		96 275)		396 .275)	3.70	(0.537)
ic	으 등 Beam length 20.32 cm (8.0 in.)						
met.	Beam t	hickness	1.5	2 cm (0.	6 in.)		
Geometric Parameter	Beam w	idth	2.5	4 cm (1.	0 in.)		

 $<sup>^{^{</sup>ar{\mathsf{M}}}}$ M  ${\scriptstyle{\circ}}$  multimodular, B  ${\scriptstyle{\circ}}$  bimodular, U  ${\scriptstyle{\circ}}$  unimodular.

Table 2 Summary of cases considered

CASE NO.	BOUNDARY CONDITION AND LOAD POSITION	CASE NO.	BOUNDARY CONDITION AND LOAD POSITION
1	Topic Topic	3	
2	<b>A</b>	4	

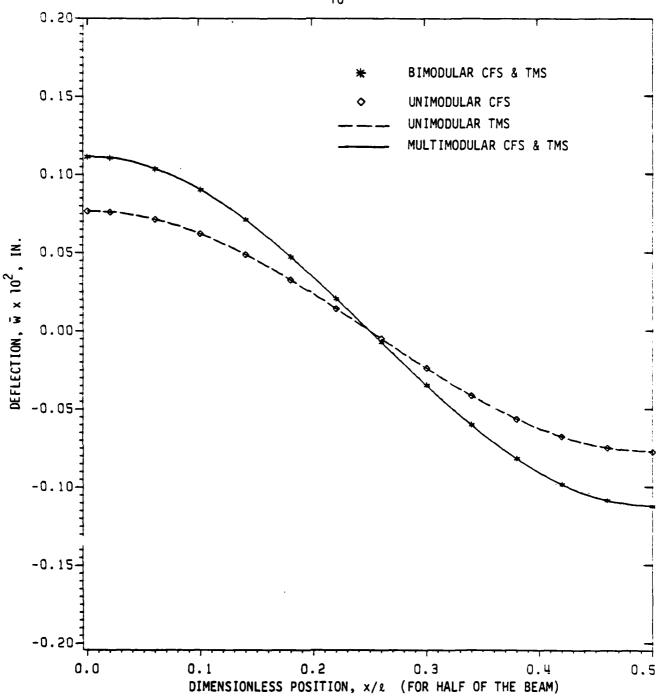


Fig. 3 Comparison among multimodular, bimodular, and unimodular deflection distribution for closed-form and transfer-matrix solutions of guided-guided aramid-cord rubber beam (f = 100 Hz)

Table 3 Comparison between CFS\* and TMS for an aramid-cord rubber beam (case 1) $^{+}$ , f = 100 Hz

x/l	N x 10	) <sup>2</sup> , 1 <u>b</u>	Qχ	10, 1b	M,	lb-in.
	CFS	TMS	CFS	TMS	CFS	TMS
0.00	-0.169	-0.164	0.000	0.000	0.203	0.203
0.02	-0.168	-0.162	-0.198	-0.200	0.201	0.202
0.06	-0.157	-0.152	-0.585	-0.587	0.189	0.189
0.10	-0.137	-0.132	-0.935	-0.937	0.164	0.165
0.14	-0.108	-0.104	-1.226	-1.228	0.129	0.130
0.18	-0.072	-0.069	-1.141	-1.143	0.086	0.087
0.22	-0.032	-0.031	-1.565	-1.566	0.038	0.038
0.26	0.011	0.010	-1.590	-1.591	-0.013	-0.013
0.30	0.052	0.050	-1.514	-1.516	-0.063	-0.063
0.34	0.091	0.087	-1.344	-1.346	-0.109	-0.109
0.38	0.123	0.119	-1.089	-1.091	-0.148	-0.148
0.42	0.148	0.143	-0.766	-0.768	-0.178	-0.178
0.46	0.164	0.158	-0.394	-0.396	-0.196	-0.197
0.50	0.169	0.163	0.000	0.000	-0.203	-0.204
0.54	0.164	0.158	0.394	0.396	-0.196	-0.197
0.58	0.148	0.143	0.766	0.768	-0.178	-0.178
0.62	0.123	0.119	1.089	1.091	-0.148	-0.148
0.66	0.091	0.087	1.344	1.346	-0.109	-0.109
0.70	0.052	0.050	1.514	1.516	-0.063	-0.063
0.74	0.011	0.010	1.590	1.591	-0.013	-0.013
0.78	-0.032	-0.031	1.565	1.566	0.038	0.038
0.82	-0.072	-0.069	1.141	1.143	0.086	0.087
0.86	-0.108	-0.104	1.226	1.228	0.129	0.130
0.90	-0.137	-0.132	0.935	0.937	0.164	0.165
0.94	-0.157	-0.152	0.585	0.587	0.189	0.189
0.98	-0.168	-0.162	0.198	0.200	0.201	0.202
1.00	-0.169	-0.164	0.000	0.000	0.203	0.203

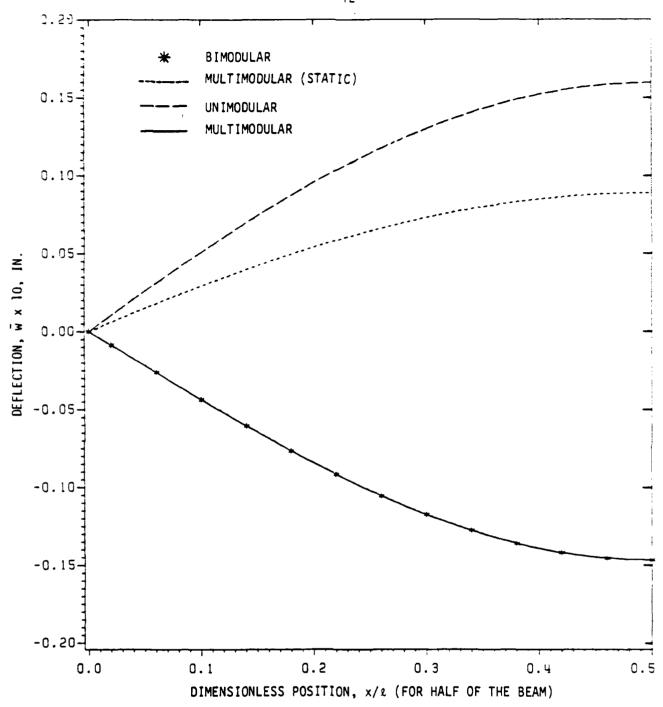


Fig. 4 Comparison among multimodular, bimodular, and unimodular deflection distribution for transfer-matrix solution of hinged-hinged, aramid-cord rubber beam (f = 100 Hz)

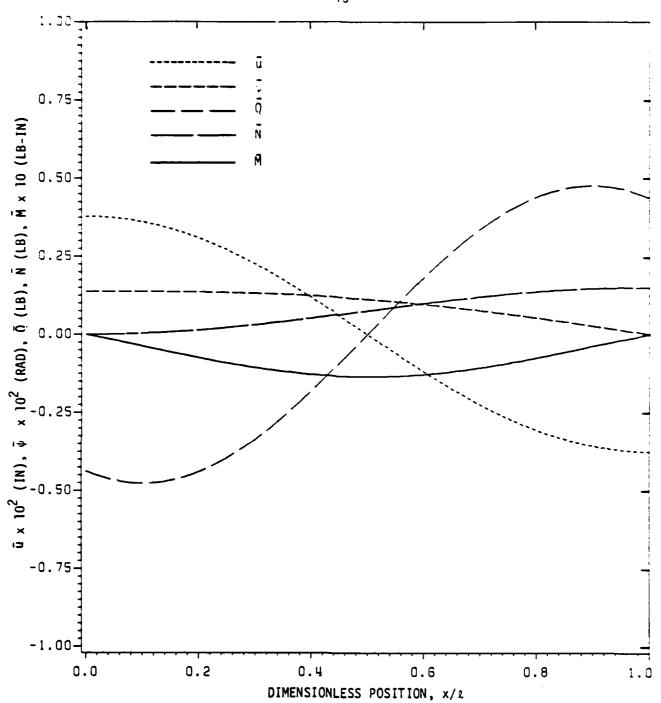


Fig. 5 Transfer-matrix solution of hinged-hinged, aramid-cord rubber beam (f = 100 Hz)

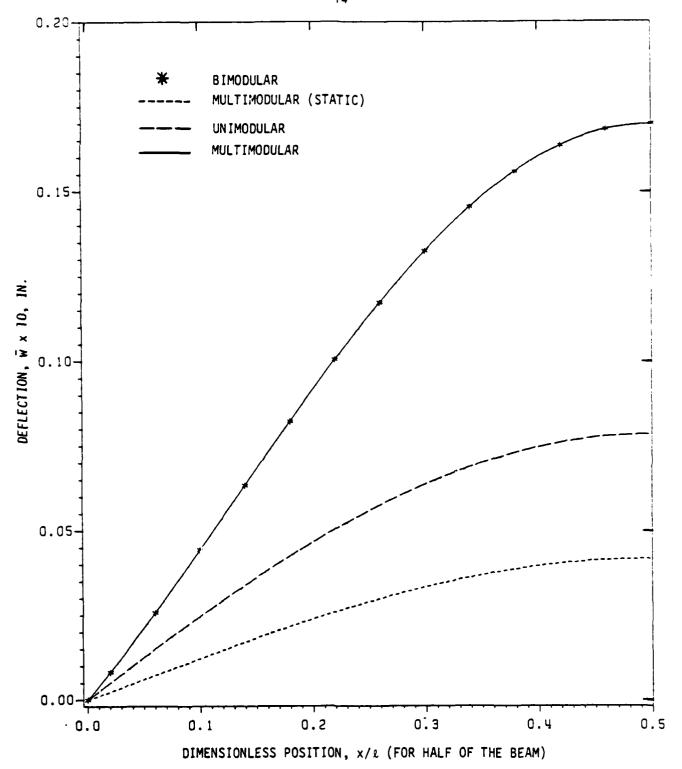


Fig. 6 Comparison among multimodular, bimodular, and unimodular deflection distribution for transfer-matrix solution of clamped-clamped aramid-cord rubber beam (f = 100 Hz)

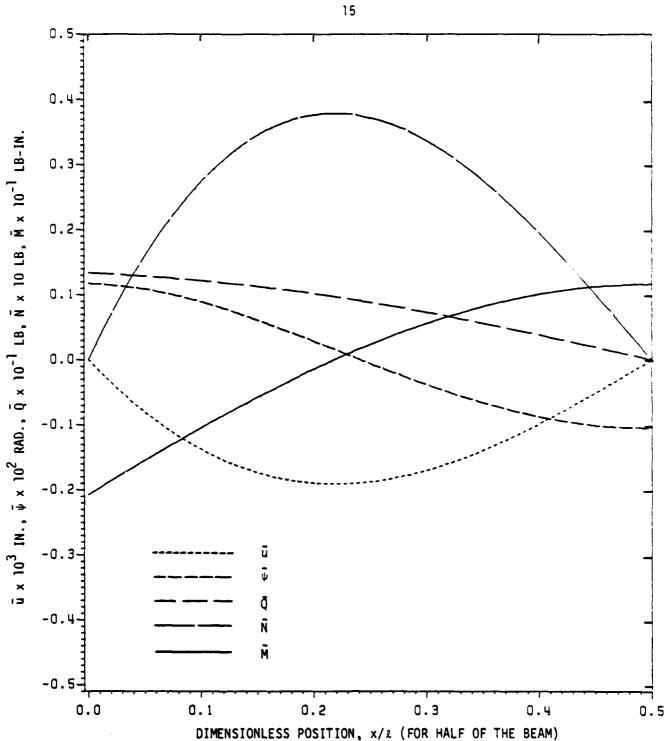


Fig. 7 Transfer-matrix solution of clamped-clamped, aramid-cord rubber beam (f = 100 Hz)

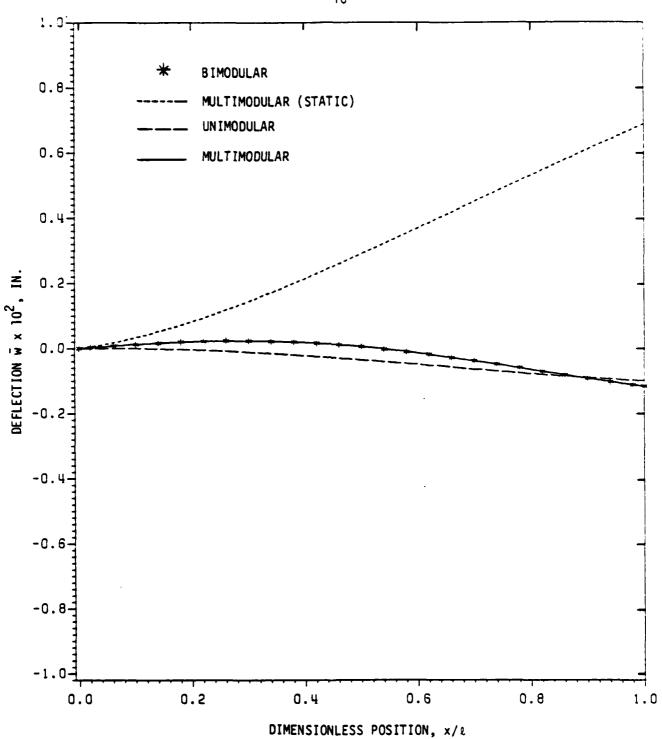


Fig. 8 Comparison among multimodular, bimodular, and unimodular deflection distribution for transfer-matrix solution of clamped-free aramid-cord rubber beam (f = 100 Hz)

modular beams are between their first and second modes. The explanation for this is that, for this case (2), most of the layers of the beam are under compression and since  $E_1^C$ ,  $E_2^C$ , and  $E_b^C$  are much smaller than E (see Table 1), then the unimodular beam is stiffer than the other two. Therefore, the fundamental frequency of the unimodular beam is higher than those of the bimodular and multimodular beams.

Also, the distributions of  $\tilde{u}$  (axial displacement),  $\tilde{\psi}$  (bending slope),  $\tilde{N}$  (axial force),  $\tilde{Q}$  (transverse shear force), and  $\tilde{M}$  (bending moment) are shown graphically in Figures 5, 7, and 9 for f taken to be 100 Hz. Note that for cases 1 and 3 this frequency is less than the fundamental frequency, whereas for cases 2 and 4, the frequency of 100 Hz is in the range of the first and second modes. The first three mode shapes of a clamped-free beam of multimodular material is investigated in Fig. 10. For this case, the natural frequencies associated with the first three modes are  $f_1$  = 30.9 Hz,  $f_2$  = 131.1 Hz, and  $f_3$  = 278.5 Hz.

Finally, by rewriting the equations of motion in a new form  $(N_{,x} = P_{||} u_{,tt}, M_{,x} = Q_{||} u_{,tt}, Q_{,x} = P_{||} u_{,tt}, Q_{,x} =$ 

### 6 CONCLUSIONS

An analysis of forced vibration of a thick beam with a rectangular cross section and made of "multimodular" material is presented. In this study, numerical results obtained by both the closed-form and transfer-matrix methods are given for a beam made of aramid-cord rubber.

Comparisons are made on one hand between closed-form and transfer-matrix results and on the other hand, among unimodular, bimodular, and multimodular

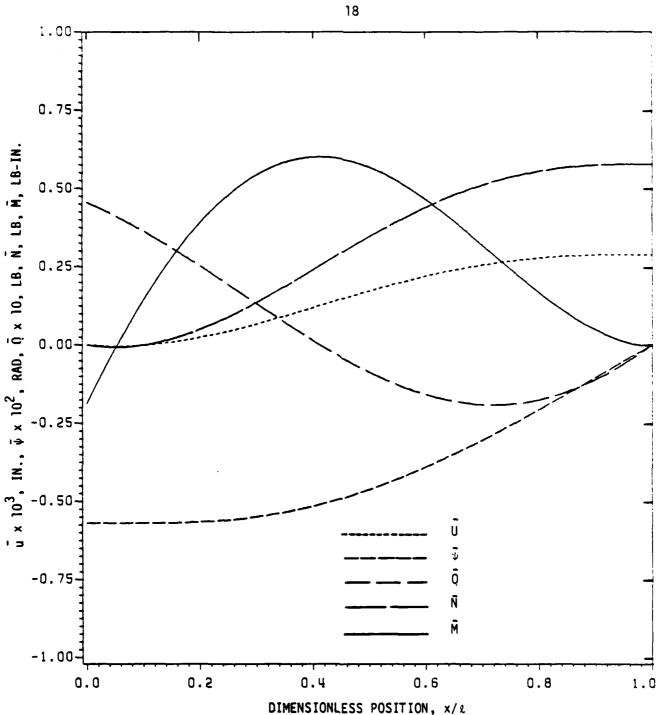


Fig. 9 Transfer-matrix solution of clamped-free aramid-cord rubber beam (f = 100 Hz)

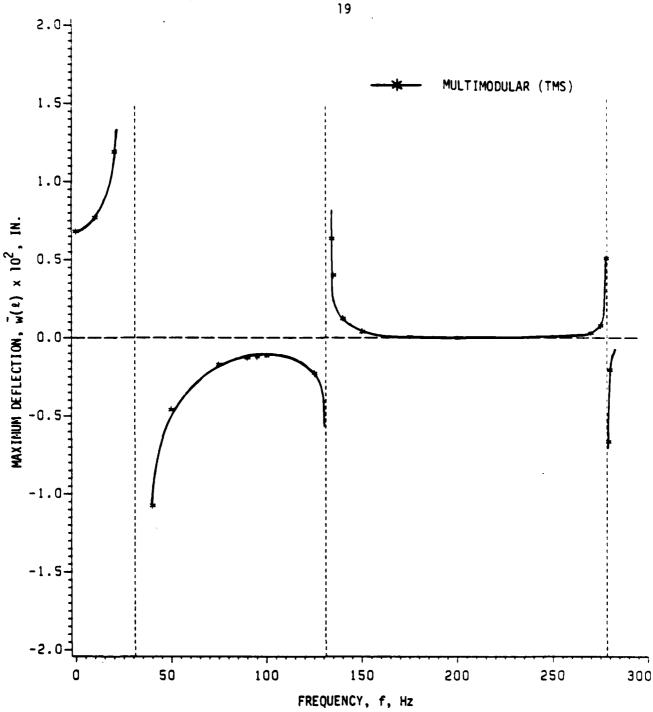


Fig. 10 Investigation of the first three mode shapes of thick multimodular, clamped-free aramid-cord rubber beam with rectangular cross section.

Table 4 Effect of translatory and rotatory inertia coefficients on axial force for a thick multimodular cantilever beam (f = 100 Hz)

	Axial force, lb x 10 <sup>3*</sup>				
x/L	I,P <sub>1</sub> ,P <sub>2</sub> ≠0	I=0 P <sub>1</sub> &P <sub>2</sub> ≠0	I&P <sub>1</sub> =0 P <sub>2</sub> ≠0	P <sub>1</sub> =0 I&P <sub>2</sub> ≠0	I,P <sub>1</sub> ,P <sub>2</sub> =0
	Full Theory				Static
0.00	11.74	11.76	-0.1399	-0.1621	-0.0699
0.02	11.49	11.51	-0.1399	-0.1621	-0.0699
0.06	10.56	10.58	-0.1399	-0.1621	-0.0699
0.10	8.90	8.92	-0.1399	-0.1621	-0.0699
0.14	6.69	6.73	-0.1399	-0.1621	-0.0699
0.18	4.15	4.19	-0.1399	-0.1621	-0.0699
0.22	1.44	1.48	-0.1399	-0.1621	-0.0699
0.26	-1.26	-1.21	-0.1399	-0.1621	-0.0699
0.30	-3.82	-3.76	-0.1399	-0.1621	-0.0699
0.34	-6.09	-6.03	-0.1399	-0.1621	-0.0699
0.38	-7.97	-7.91	-0.1399	-0.1621	-0.0699
0.42	-9.38	-9.31	-0.1399	-0.1621	-0.0699
0.46	-10.25	-10.18	-0.1399	-0.1621	-0.0699
0.50	-10.54	-10.47	-0.1399	-0.1621	-0.0699

<sup>\*1 1</sup>b = 4.448 newtons

models. These results show a considerable difference between the unimodular and bimodular models and a slight difference between the bimodular and multimodular models. Therefore, although a four-segment model is a better approximation, the two-segment approximation gives nearly the same results. This proves that the bimodular model precision is a good approximation.

The values of the first three mode shapes for the clamped-free case are presented. Finally, the effects of axial translatory and rotatory inertia coefficients on axial force for a clamped-clamped beam are discussed.

The transfer-matrix method is found to be very effective in terms of computational time and also in terms of the accuracy of results, which agree very well with the closed-form solution.

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APPENDIX A: FITTING MINIMIZED CURVES TO THE STRESS-STRAIN CURVE

### 1. Multimodular Case

Consider the nonlinear stress-strain curve shown in Fig. A.l. For any arbitrary point  $(\varepsilon^{\mathbf{t}}, \sigma^{\mathbf{t}})$  in the tension region  $(\varepsilon \geq 0)$ , there are two straight lines such that

$$g(\varepsilon) = \begin{cases} (\sigma^{t}/\varepsilon^{t})\varepsilon \\ [(\sigma^{t} - \sigma_{f}^{t})/(\varepsilon^{t} - \varepsilon_{f}^{t})](\varepsilon - \varepsilon_{f}^{t}) + \sigma_{f}^{t} \end{cases}$$
(A.1)

The equation of a stress-strain curve is

$$\sigma(\varepsilon) = K \varepsilon^{\mathsf{n}} \quad ; \quad \varepsilon > 0$$
 (A.2)

where K and n are constants depending on the material. To find the proper "break point" ( $\varepsilon^{t}$ , $\sigma^{t}$ ), the area between the approximated curve  $g(\varepsilon)$  and the actual experimental curve  $\sigma(\varepsilon)$  has to be minimized. The mentioned area can be expressed

$$A = \left| \int_{0}^{\varepsilon} [g_{1}(\varepsilon) - \sigma(\varepsilon)] d\varepsilon \right| + \left| \int_{\varepsilon}^{\varepsilon} f [g_{2}(\varepsilon) - \sigma(\varepsilon)] d\varepsilon \right|$$
(A.3)

Substitution of equations (A.1) and (A.2) into equation (A.3) and performing the integrations gives

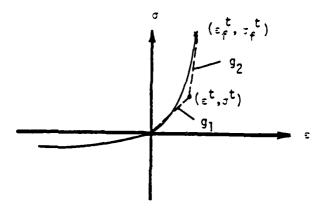


Fig. A.1 Multimodular model.

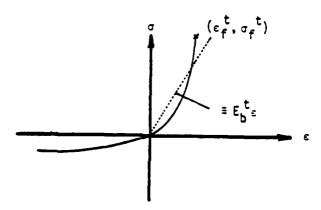


Fig. A.2 Bimodular model.

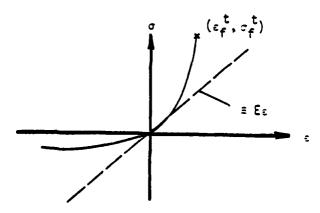


Fig. A.3 Unimodular model.

$$A = \left| \frac{1}{2} \sigma^{t} \varepsilon^{t} - \frac{K}{n+1} (\varepsilon^{t})^{n+1} \right| + \left| \frac{1}{2} (\sigma_{f}^{t} + \sigma^{t}) (\varepsilon_{f}^{t} - \varepsilon^{t}) - \frac{K}{n+1} [(\varepsilon_{f}^{t})^{n+1} - (\varepsilon^{t})^{n+1}] \right|$$
(A.4)

By searching in the region of  $\Omega \equiv (0, \varepsilon_t^f) \times (0, \sigma_t^f)$ , one is able to find a point  $(\varepsilon^t, \sigma^t)$  such that A is minimized locally. Note that a few other methods (e.g., least-squares method) have been tried but it turned out that the absolute minimum point was outside of the region  $\Omega$ .

### 2. Bimodular Case

For this case, the least-squares method has been used. As shown in Fig. A.2, there is a line such that

$$I = \int_{0}^{\varepsilon f} [E_{b}^{t} \varepsilon - K \varepsilon^{n}]^{2} d\varepsilon$$
 (A.5)

can be minimized in  $\Omega$ . Here,  $E_b^{\ t}$  is the slope of that line. By taking the derivative of equation (A.5) and equating it to zero, one has

$$\frac{dI}{dE_b^{t}} = 2 \int_{0}^{\varepsilon_{f}} [E_b^{t} \varepsilon - K \varepsilon^{n}] \varepsilon d\varepsilon = 0$$
 (A.6)

By solving equation (A.6) for  $E_h^t$ , one obtains

$$E_{h}^{t} = \frac{3K}{n+2} \left( \varepsilon_{f}^{t} \right)^{n-1} \tag{A.7}$$

For example, for aramid-rubber in the tension region, the following parameters are found (the other constants are listed in [10]):

$$n_t = 1.22$$
 ;  $K_t = 1.1 \times 10^6 \text{ psi}$  ;  $\epsilon_f^t = 0.029$   
 $E_b^t = \frac{(3)(1.1 \times 10^6)}{1.22 + 2} (0.029)^{1.22 - 1} = 0.47 \times 10^6 \text{ psi}$ 

An analogous calculation can be applied for the compression side of the bend, i.e.,  $E_b^c$  can be found, provided that  $K_c$ ,  $n_c$ , and  $\epsilon_f^c$  are known.

A similar approach can be used to obtain a best-fit single straight line ("unimodular" approximation) [5]; see Fig. A.3.

APPENDIX B: THE BEAM STIFFNESSES FOR RECTANGULAR-SECTION BEAMS OF MULTIMODULAR MATERIALS

For the assumed four-segment model, there are two different bending cases in general, convex downward and concave downward bending. In convex downward bending, the top layer of a beam is in compression and the bottom layer in tension.

A stress distribution for convex downward bending is shown in Fig. 2. As one might notice, the location of  $z_n$ ,  $a_c$  and  $a_t$  fall within the depth of the beam (this is case is the most general case). Substitution of equation (1) into equation (8) and using equations (3), (4), and (5) leads to

$$N = \int_{-h/2}^{a_{c}} [\kappa E_{1}^{c}(a_{c} - z_{n}) + \kappa E_{2}^{c}(z - a_{c})]dz + \int_{a_{c}}^{z_{n}} \kappa E_{1}^{c}(z - z_{n})dz + \int_{z_{n}}^{a_{t}} \kappa E_{1}^{t}(z - z_{n})dz$$

$$+ \int_{a_{t}}^{h/2} [\kappa E_{1}^{t}(a_{t} - z_{n}) + \kappa E_{2}^{t}(z - a_{t})]dz$$
(B.1)

and

$$M = \int_{-h/2}^{a_{c}} [\kappa E_{1}^{c} (a_{c} - z_{n}) + \kappa E_{2}^{c} (z - a_{c})] z \, dz + \int_{a_{c}}^{z_{n}} \kappa E_{1}^{c} (z - z_{n}) z \, dz$$

$$+ \int_{z_{n}}^{a_{t}} \kappa E_{1}^{t} (z - z_{n}) z \, dz + \int_{a_{t}}^{h/2} [\kappa E_{1}^{t} (a_{t} - z_{n}) + \kappa E_{2}^{t} (z - a_{t})] z \, dz \qquad (B.2)$$

Equations (B.1) and (B.2) can be written in the following form

$$N = (-\kappa z_n) \{ \left[ \int_{-h/2}^{a_c} E_2^{c} dz + \int_{a_c}^{z_n} E_1^{c} dz + \int_{z_n}^{a_t} E_1^{t} dz + \int_{a_t}^{h/2} E_2^{t} dz \right] + \left[ -\int_{-h/2}^{a_c} E_2^{c} dz + \int_{-h/2}^{a_c} E_1^{c} dz + \int_{a_t}^{h/2} E_1^{t} dz - \int_{a_t}^{h/2} E_2^{t} dz \right] \}$$

$$+ (<)\{\left[\int_{-h/2}^{a_{c}} E_{2}^{c} z dz + \int_{a_{c}}^{z} E_{1}^{c} z dz + \int_{a_{c}}^{a_{t}} E_{1}^{t} z dz + \int_{a_{t}}^{h/2} E_{2}^{t} a_{t} dz - \int_{a_{t}}^{h/2} E_{2}^{t} a_{t} dz\right]\}$$

$$+ \left[-\int_{-h/2}^{a_{c}} E_{2}^{c} a_{c} dz + \int_{-h/2}^{a_{c}} E_{1}^{c} a_{c} dz + \int_{a_{t}}^{h/2} E_{1}^{t} a_{t} dz - \int_{a_{t}}^{h/2} E_{2}^{t} a_{t} dz\right]\}$$
(B.3)

$$M = (-\langle z_n \rangle) \{ \left[ \int_{-h/2}^{a_c} E_2^c z \, dz + \int_{a_c}^{z_n} E_1^c z \, dz + \int_{z_n}^{a_t} E_1^t z \, dz + \int_{a_t}^{h/2} E_2^t z \, dz \right]$$

+ 
$$[-\int_{-h/2}^{a_c} E_2^{c}z dz + \int_{-h/2}^{a_c} E_1^{c}z dz + \int_{a_t}^{h/2} E_1^{t}z dz - \int_{a_t}^{h/2} E_2^{t}z dz]$$

+ (<) {[
$$\int_{-h/2}^{a_c} E_2^{c} z^2 dz + \int_{a_c}^{z_n} E_1^{c} z^2 dz + \int_{z_n}^{a_t} E_1^{t} z^2 dz + \int_{a_t}^{h/2} E_2^{t} z^2 dz]$$

$$+ \left[ - \int_{-h/2}^{a_{c}} E_{2}^{c} a_{c} dz + \int_{-h/2}^{a_{c}} E_{1}^{c} a_{c} dz + \int_{a_{t}}^{h/2} E_{1}^{t} a_{t} dz - \int_{a_{t}}^{h/2} E_{2}^{t} a_{t} dz \right]$$
 (B.4)

Combining equations (7) and (9), one gets

$$N = (-\langle z_n \rangle A' + \langle B' \rangle$$
 (B.5)

$$M = (-\kappa z_n)B'' + \kappa D'$$
 (B.6)

Comparison of equations (B.3) and (B.5) with equations (B.4) and (B.6) and considering equations (10), one finds that

$$C_A^N = \int_{-h/2}^{a_c} (E_1^c - E_2^c) dz + \int_{a_t}^{h/2} (E_1^t - E_2^t) dz$$

$$C_B^N = \int_{-h/2}^{a_c} (E_1^c - E_2^c) a_c dz + \int_{a_t}^{h/2} (E_1^t - E_2^t) a_t dz$$

$$C_B^{H} = \int_{-h/2}^{a_c} (E_1^c - E_2^c)z dz + \int_{a_c}^{h/2} (E_1^t - E_2^t)z dz$$

$$C_{D}^{M} = \int_{-h/2}^{a_{C}} (E_{1}^{C} - E_{2}^{C}) a_{C}^{z} dz + \int_{a_{t}}^{h/2} (E_{1}^{t} - E_{2}^{t}) a_{t}^{z} dz$$
(B.7)

Eight possible cases may occur depending on the location of  $z_n$ ,  $a_c$ , and  $a_t$ . These cases have been analyzed as the same as the general case as follows (for convex downward bending).

There are seven more possible cases which have been discussed in Ref. [5]. However, the case explained here is the most general case.

### APPENDIX C: TRANSFER-MATRIX FORMULATION

Under harmonic excitation, the steady-state-response displacements u, w, and , are also harmonic in time. Therefore, equations (11) can be written as

$$\vec{N}_{,x} = -\Omega^2 P \vec{u} - \Omega^2 R \vec{\psi} ; \quad \vec{Q}_{,x} = -\Omega^2 P \vec{w} - q(x,t)$$

$$\vec{M}_{,x} - \vec{Q} = -\Omega^2 R \vec{u} - \Omega^2 I \vec{\psi}$$
(C.1)

where all of the barred quantities are amplitudes, i.e.,  $N(x,t) = \bar{N}(x) \sin \Omega t$ , etc. The continuity at each station implies

$$\vec{u}_i^R = \vec{u}_i^L$$
 ,  $\vec{w}_i^R = \vec{w}_i^L$  ,  $\vec{\psi}_i^R = \vec{\psi}_i^L$  (C.2)

(R and L denote right and left, respectively)

Also, equation (C.1) in finite-differential form for each station i is

$$\bar{N}_{i}^{R} = \bar{N}_{i}^{L} - \Omega^{2}P\bar{u}_{i}^{L} ; \quad \bar{Q}_{i}^{R} = \bar{Q}_{i}^{L} - \Omega^{2}P\bar{w}_{i}^{L} - \bar{Q}_{i}$$

$$\bar{M}_{i}^{R} = \bar{M}_{i}^{L} - \Omega^{2}R\bar{u}_{i}^{L} - \Omega^{2}I\bar{\psi}_{i}^{L}$$
(C.3)

where  $\mathbf{Q}_{\mathbf{i}}$  is the concentrated load amplitude at station i.

Equations (A.2) and (A.3) are written in matrix notation as

$$\begin{cases}
\bar{u} \\
\bar{w} \\
\bar{v} \\
\bar{v} \\
\bar{N} \\
\bar{Q} \\
\bar{M} \\
1
\end{cases} =
\begin{cases}
1 & 0 & 0 & 0 & 0 & 0 & 0 \\
0 & 1 & 0 & 0 & 0 & 0 & 0 \\
0 & 0 & 1 & 0 & 0 & 0 & 0 \\
-\Omega^2 P & 0 & -\Omega^2 R & 1 & 0 & 0 & 0 \\
0 & -\Omega^2 P & 0 & 0 & 1 & 0 & \bar{Q}_1 \\
-\Omega^2 R & 0 & -\Omega^2 I & 0 & 0 & 1 & 0 \\
0 & 0 & 0 & 0 & 0 & 0 & 1
\end{cases} \quad
\begin{pmatrix}
\bar{u} \\
\bar{w} \\
\bar{v} \\
\bar{N} \\
\bar{Q} \\
\bar{M} \\
1
\end{pmatrix}_{1}$$
(C.4)

or

$$[S]_{i}^{R} = [T_{S}]_{i}^{L} [S]_{i}^{L}$$

where  $[T_S]_i$  represents station matrix at station i. In matrix notation the equilibrium equation for each field under a distributed load q(x) is

$$\begin{cases} u \\ w \\ \psi \\ N \\ 1 \end{cases} = \begin{bmatrix} 1 & 0 & 0 & \frac{B^{1}\Delta\ell}{\gamma} & \frac{B^{1}(\Delta\ell)^{2}}{2\gamma} & \frac{-D^{1}\Delta\ell}{\gamma} & \frac{-B^{1}\Delta\ell}{2\gamma} & \kappa_{m} \\ 0 & 1 & -\Delta\ell & \frac{A^{1}(\Delta\ell)^{2}}{2\gamma} & \left[\frac{\Delta\ell}{S} + \frac{A^{1}(\Delta\ell)^{3}}{4\gamma}\right] & \frac{-B^{1}(\Delta\ell)^{2}}{2\gamma} & \frac{-A^{1}(\Delta\ell)^{2}}{4\gamma} & \kappa_{m} - \frac{\Delta\ell}{2S} & \kappa_{q} \\ 0 & 0 & 1 & \frac{-A^{1}\Delta\ell}{\gamma} & \frac{-A^{1}(\Delta\ell)^{2}}{2\gamma} & \frac{B^{1}\Delta\ell}{\gamma} & \frac{A^{1}\Delta\ell}{2\gamma} & \kappa_{m} \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & -\kappa_{q} \\ 0 & 0 & 0 & 0 & \Delta\ell & 1 & -\kappa_{m} \\ 1 \\ 1 \\ 1 \\ 1 \end{bmatrix}_{i+1}$$

(C.5)

where

$$\gamma = B'B'' - A'D'$$
;  $K_q = \int_0^{\Delta \ell} q(\xi)d\xi$ ;  $K_m \int_0^{\Delta \ell} \xi q(\xi)d\xi$  (C.6)

Values of  ${\rm K_m}$  and  ${\rm K_q}$  for various loadings are listed in Table 5. Equation (C.6) also can be written as

Table 5 Values of  $K_{\overline{m}}$  and  $K_{\overline{q}}$  for various loadings

Type of Loading	K <sub>m</sub>	Kg
Uniform Load q(x) = q <sub>o</sub>		
→ ×	q <sub>0</sub> (Δl) <sup>2</sup> /2	g <sub>o</sub> ∆ℓ
Cosine Load $q(x) = q_0 \cos \frac{n\pi}{\ell} x$	$\frac{q_0^{\ell}}{n\pi} \left[ -\frac{\ell}{n\pi} \left( \cos \frac{n\pi}{\ell} x_j \right) \right]$	$\frac{q_0^{\ell}}{n\pi}$ (sin $\frac{n\pi}{\ell} \times_j$
×	- $\cos \frac{n\pi}{\ell} x_{j-1}$ ) - $\ell \sin \frac{n\pi}{\ell} x_{j-1}$ ]	- $\sin \frac{n\pi}{\ell} x_{j-1}$ )

$$\begin{bmatrix} S \end{bmatrix}_{i+1}^{L} = \begin{bmatrix} T_j \end{bmatrix}_{i}^{R}$$
(C.7)

The matrix  $[T_j]_i$  is called the field matrix.

APPENDIX D: COMPUTATION OF  $z_n$ ,  $a_c$ , AND  $a_t$ 

For multimodular beams, the following equation is not sufficient to determine the neutral-surface location  $\boldsymbol{z}_n$ 

$$z_n = \frac{B'M - D'N}{A'M - B''N} \tag{D.1}$$

even for cases where N = 0

$$z_n = B'/A' \tag{D.2}$$

Two more equations are needed for computing  $z_n$  because the stiffnesses are not only dependent on  $z_n$  but they are functions of  $a_c$  and  $a_t$  as well. By comparison of Figures 1 and 2, one can get (for the convex downward case)

$$a_{c} = (\varepsilon_{1}^{c}/\varepsilon_{f}^{c}) (h/2 + z_{n}) - z_{n}$$
 (D.3)

$$a_t = (\epsilon_1^t/\epsilon_f^t) (h/2 - z_n) + z_n$$
 (D.4)

For the concave downward case

$$a_c = (\epsilon_1^c / \epsilon_f^c) (h/2 - z_n) + z_n$$
 (D.5)

$$a_{t} = (\varepsilon_{1}^{t}/\varepsilon_{f}^{t}) (h/2 + z_{n}) - z_{n}$$
 (D.6)

The system of nonlinear equations (D.1), (D.3), and (D.4) for the convex downward case, or equations (D.1), (D.5), and (D.6) for the concave downward case, can be solved by using iteration of the Gauss-Seidel type.

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OU denotes the University of Oklahoma; VPI denotes Virginia Polytechnic Institute and State University.

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Project Rept. No.	Issuing University Rept. No.	Report Title	Author(s)
29	VPI 82.19	Three-Dimensional Finite Element Analysis of Layered Composite Structures	W.C. Chao, N.S. Putcha, J.N. Reddy
30	OU 82-5	Analyses of Beams Constructed of Nonlinear Materials Having Different Behavior in Tension and Compression	C.W. Bert & F. Gordaninejad
31	VPI 82.31	Analysis of Layered Composite Plates by Three- Dimensional Elasticity Theory	J.N. Reddy & T. Kuppusamy
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### 18. SUPPLEMENTARY NOTES

This paper is to be presented at the 19th ASME Conference on Mechanical Vibration and Noise, Dearborn, MI, Sept. 12-14, 1983.

19. KEY WORDS (Continue on reverse side if necessary and identify by block number)

Beams, bending theory, bending-stretching coupling, bimodular materials, multimodular materials, nonlinear materials, Timoshenko beam theory, transfer-matrix method, vibration.

20. ABSTRACT (Continue on reverse side if necessary and identify by block mamber)

This paper presents a transfer-matrix analysis for determining the sinusoidal vibration response of thick, rectangular-cross-section beams made of "multimodular materials" (i.e., materials which have different elastic behavior in tension and compression, with nonlinear stress-strain curves approximated as piecewise linear). An experimentally determined stress-strain curve for aramid-cord rubber is approximated by four straight-line segments (two segments in tension and two segments in compression). To validate (over)

### 20. Abstract - Cont'd

the transfer-matrix results, a closed-form solution is also presented for the special case in which the neutral-surface location is uniform along the length of the beam. Also, comparisons are made among multimodular, bimodular (two line segments), and unimodular models. Numerical results for axial displacement, transverse deflection, bending slope, bending moment, transverse shear and axial forces, and the location of the neutral surface are presented for the multimodular model. Effects of translatory and rotatory inertia coefficients on axial force are investigated for a clamped-clamped beam. Moreover, natural frequencies associated with the first three modes of a clamped-free beam are presented. Transfer-matrix results agree very well with the closed-form results for the corresponding material model (one, two, or four segments).

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